Old Currituck Jail and Courthouse
Exterior Restoration – Phase One
Currituck, NC
Bidding Tuesday, March 5 at 3:00 PM

Addendum No. One

A Pre-Bid meeting was held at the site on Monday, February 18, 2013 at 10:30 AM. A list of attendees is attached.

Project Manual/Specifications

Instructions to Bidders

No Bid Bond is required.

No Minority Business Participation documentation is required.

General Conditions

Performance and Payment Bonds are required from the successful bidder.

Section 01 7000 – Execution and Closeout Requirements

3.04 Alterations. Add the following: "J. No testing for lead-based paint has been done on either building. Given the age of the buildings, the Contractor should assume that any existing paint encountered contains lead, and take care to protect workers and the public from hazards, and to dispose of painted waste accordingly."

Section 06 2000 – Finish Carpentry

2.02 Lumber Materials. Atlantic White Cedar may be best quality STK grade with no sap wood.

Section 09 9000 – Painting and Coating

2.03 Paint Systems – Exterior, B. Paint CE-OP-2L is the wash coating referred to on Drawing Sheet A-200 General Masonry Notes as Alternate GC – 3. This is for application to the exterior brick masonry of the Old Jail only.

Drawings

A-100 Floor Plans

See Note: "Temporarily support existing stair and second floor structure during new floor construction." It is the contractor's option as to how the floor and stair are to be supported, or whether to dismantle and reconstruct these elements. The stair and second floor structure must be in place and useable at the completion of this project.

A-200 Elevations

Add the following note to 2/A-200. "Replacement brick must match original brick in size, texture and color." It is likely that different replacement brick will be required for the Old Jail and the Courthouse.

S103 Structural Details

See revised Sheet S103 attached. The revision is to Detail 07 Rafter Connection Detail.

HagerSmith Design, PA
22 February 2013

MEETING SIGN-IN SHEET

PROJECT NAME:	Currituck Jail and Courthouse		DATE:	18-Feb-13
NAME:	**REPRESENTING:**	**PHONE:**	**FAX:**	**E-MAIL:**
David Black, AIA	**HagerSmith Design, PA**	**919-838-5105**	**919-828-4050**	dblack@hagersmith.com
Michael Roberts	AR Chesson Const	252-338-9171	252-338-9172	michael@archesson.com
David Deyo	"	"	"	daviddeyo@archesson.com
Guy Drake	Drake Welding Inc	757-393-9630	757-393-9635	gdrakeweldmg@gmail.com
ADAM SUTTON	WATERFRONT MARINE CONST.	757-468-1500		
HARRY DAVIS	SUSSEX DEVELOPMENT	757-422-2400	757-422-0298	hdavis@sussexdevelopment.com
Jamie Kirby	White Electric Co	252-491-0123	252-491-1007	jkirby@whiteelectric.info
Eric Ferguson	Progressive Contracting Co	(252) 482-4174 ext 361	(252) 482-4897	eferguson@progressivecc.c
LARRY WALES	WOODCRAFTER OF VA	757 425 9494	757 491 2090	larry@woodcrafterva.com
Randy Guyan	R.C. Guyan Const.	583-3316		T. Guyan [illegible]
Mandy Cervantes	Waldt Construction	(252) 491-2204	(252) 491-8186	mandy@waldt.com
Ronnie Cooper	Narrow Shores	(252) 207-6605	252 453-0384	narrowshores@yahoo.com
MICHELLE PERRY	CURRITUCK CO.	252-232-6034	252-232-3298	MICHELLE.PERRY@CURRITUCKCOUNTYNC.GOV
ERIC WEATHERLY	"	252-232-6035	"	eric.weatherly@currituckcountync.gov

GENERAL STRUCTURAL NOTES

GENERAL

THESE DRAWINGS, AS INSTRUMENTS OF PROFESSIONAL SERVICE, ARE THE PROPERTY OF LYSAGHT & ASSOCIATES, P.A., FOR USE SOLELY WITH THIS PROJECT AND SHALL NOT BE REPRODUCED FOR OTHER PURPOSES.

THE PROFESSIONAL ENGINEER WHOSE SEAL APPEARS ON THESE DRAWINGS IS THE PROJECT STRUCTURAL ENGINEER-OF-RECORD (SER) WHO BEARS LEGAL RESPONSIBILITY FOR THE PERFORMANCE OF THE STRUCTURAL FRAMING RELATING TO PUBLIC HEALTH, SAFETY AND WELFARE. NO OTHER PARTY, WHETHER OR NOT A PROFESSIONAL ENGINEER, MAY COMPLETE, CORRECT, REVISE, DELETE OR ADD TO THESE CONSTRUCTION DOCUMENTS OR PERFORM INSPECTIONS OF THE WORK WITHOUT THE WRITTEN PERMISSION OF THE SER.

WHENEVER EXISTING CONSTRUCTION IS RENOVATED THERE WILL ALWAYS BE SOME COSMETIC DEFECTS DUE TO THE AGE OF THE BUILDING THAT WON'T BE CORRECTED DURING THE RENOVATION. THESE DEFECTS INCLUDE SAGGING FLOORS, MINOR CRACKS IN MASONRY WALLS, CRACKS IN SHEETROCK OR PLASTER THAT IS LEFT IN PLACE, ETC. THIS IS TO BE EXPECTED BY THE OWNER UNLESS OTHERWISE NOTED ON THE DRAWINGS.

USE STRUCTURAL DRAWINGS IN CONJUNCTION WITH JOB SPECIFICATIONS, AND OTHER DRAWINGS.

SECTIONS AND DETAILS SHOWN SHALL BE CONSIDERED TYPICAL FOR ALL SIMILAR CONDITIONS.

CONTRACTOR SHALL VERIFY ALL CONDITIONS IN THE FIELD AND TAKE ALL NECESSARY FIELD MEASUREMENTS.

DIMENSIONS

THE CONTRACTOR, BEFORE STARTING ANY WORK, SHALL CHECK ALL DIMENSIONS GIVEN ON THE STRUCTURAL DRAWINGS, RELATING TO GRID LINES, COLUMN AND WALL LOCATIONS, STRUCTURAL AND FINISHED FLOOR ELEVATIONS, MEMBER SIZES, ETC., WITH THE ARCHITECTURAL DRAWINGS AND EXISTING CONDITIONS. IF ANY DISCREPANCY IS NOTICED, IT SHALL IMMEDIATELY BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND ENGINEER AND WORK SHALL NOT COMMENCE UNTIL INSTRUCTIONS ARE RECEIVED FROM THE ENGINEER.

THE CONTRACTOR SHALL SEEK INSTRUCTION FROM THE ENGINEER FOR ANY DIMENSION NOT GIVEN OR OBTAINABLE FROM THE DRAWINGS. THE CONTRACTOR SHALL NOT USE SCALE TO OBTAIN OR VERIFY ANY DIMENSION SHOWN ON THESE DRAWINGS.

ASSUMPTIONS

FOR PURPOSES OF THESE NOTES, ASSUMPTION SHALL BE DEFINED AS " TO BELIEVE, THINK OR SUPPOSE A CONDITION TO BE TRUE." AN ASSUMPTION CANNOT BE CONFIRMED BY THE STRUCTURAL ENGINEER BECAUSE IT IS BEYOND HIS SCOPE OF SERVICES AND/OR EXPERTISE. IF THE CLIENT REQUIRES CONFIRMATION OF AN ASSUMPTION, THEN ANOTHER EXPERT MUST DO THE NECESSARY CALCULATIONS AND/OR TESTING.

THE FOLLOWING ASSUMPTIONS HAVE BEEN MADE REGARDING THE STRENGTHS OF THE VARIOUS EXISTING STRUCTURAL COMPONENTS.

1.	ALLOWABLE SOIL BEARING PRESSURE	2000	PSF
2.	CONCRETE, f'c	3000	PSI
3.	REBAR, fy	40000	PSI
4.	MASONRY COMPRESSIVE STRENGH, f'cm	750	PSI

SCOPE OF STRUCTURAL ENGINEERING SERVICES

LYSAGHT & ASSOCIATES HAS PERFORMED THE STRUCTURAL DESIGN AND PREPARED THE STRUCTURAL WORKING DRAWINGS FOR THIS RENOVATION. "CONSTRUCTION REVIEW" SERVICES ARE ALSO A PART OF OUR CONTRACT. THE CONTRACTOR MUST NOTIFY THE STRUCTURAL ENGINEER AT THE FOLLOWING STAGES OF CONSTRUCTION FOR A FIELD REVIEW OF THE WORK:

1. AFTER COMPLETION OF THE STRUCTURAL SYSTEM, BEFORE INTERIOR FINISHES ARE INSTALLED.
2. AT ANY STAGE OF CONSTRUCTION WHEN DESIGN OR CONSTRUCTION PROBLEMS ARE ENCOUNTERED.

A "CONSTRUCTION REVIEW REPORT" WILL BE SENT TO THE CONTRACTOR AND THE ARCHITECT FOLLOWING EACH FIELD TRIP.

THE STRUCTURAL ENGINEER IS RESPONSIBLE FOR THE DESIGN OF THE PRIMARY STRUCTURAL SYSTEM, EXCEPT FOR THE COMPONENTS NOTED ABOVE. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR ANY SECONDARY STRUCTURAL AND NON-STRUCTURAL SYSTEMS NOT SHOWN ON THE STRUCTURAL PLANS.

THE STRUCTURAL ENGINEER HAS NOT DONE A SUBSURFACE INVESTIGATION (HE IS NOT A SOILS SPECIALIST). THE FOUNDATION DESIGN IS BASED UPON AN ASSUMED ALLOWABLE BEARING PRESSURE AS SHOWN IN THE "FOUNDATION" STRUCTURAL NOTES. THIS ALLOWABLE BEARING PRESSURE MUST BE VERIFIED BY THE CONTRACTOR OR OWNER. IF PROBLEMS ARE ENCOUNTERED, A SOILS ENGINEER SHOULD BE RETAINED TO EVALUATE THE CONDITIONS AND RECOMMEND THE APPROPRIATE FOUNDATION SYSTEM.

THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR, AND WILL NOT HAVE CONTROL OF, CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES, OR FOR SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE CONSTRUCTION WORK; NOR WILL HE BE RESPONSIBLE FOR THE CONTRACTOR'S FAILURE TO CARRY OUT THE CONSTRUCTION WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

FIELD MEASUREMENTS AND THE VERIFICATION OF FIELD DIMENSIONS ARE NOT PART OF LYSAGHT & ASSOCIATES' RESPONSIBILITY. THE CONTRACTOR MUST CHECK ALL (ASSUMED) EXISTING CONDITIONS SHOWN ON THESE DRAWINGS FOR ACCURACY AND NOTIFY THE STRUCTURAL ENGINEER OF ANY DISCREPANCIES.

ABBREVIATIONS

AB ANCHOR BOLT
AFF ABOVE FINISH FLOOR
B/B BACK TO BACK
BOD BOTTOM OF DECK
C/C CENTER TO CENTER
CJ CONTROL OR CONSTRUCTION JOINT IN SLAB
EJ EXPANSION JOINT
EOS EDGE OF SLAB
EW EACH WAY
FF FINISH FLOOR
FFE FINISH FLOOR ELEVATION
LVL LAMINATED VENEER LUMBER
MCJ MASONRY CONTROL JOINT
NTS NOT TO SCALE
OC ON CENTER
SER STRUCTURAL ENGINEER-OF-RECORD
TOF TOP OF FOOTING
TOS TOP OF STEEL
TOJ TOP OF JOIST
UNO UNLESS NOTED OTHERWISE
VIF VERIFY IN FIELD
W.P. WORK POINT
WWF WELDED WIRE FABRIC

CODE

NORTH CAROLINA STATE BUILDING CODE, 2009 EDITION

DESIGN DATA

FLOOR LIVE LOAD	100	PSF
ROOF DEAD LOAD	20	PSF
ROOF COLLATERAL LOAD	5	PSF
ROOF LIVE LOAD	20	PSF
GROUND SNOW LOAD	15	PSF
FLAT ROOF SNOW LOAD	15	PSF
SNOW EXPOSURE FACTOR	1.0	
SNOW LOAD IMPORTANCE FACTOR	1.0	
THERMAL FACTOR	1.0	
RAIN ON SNOW SURCHARGE	5	PSF
BASIC WIND SPEED (3-SECOND GUST)	110	MPH
WIND IMPORTANCE FACTOR	1.0	
WIND EXPOSURE	C	

BUILDING FRAME SYSTEM WITH INTERMEDIATE REINFORCED MASONRY SHEAR WALLS. SIMPLIFIED ANALYSIS PROCEDURE PER SECTION 1617.5

BUILDING CODE REQUIREMENTS FOR EXISTING BUILDINGS

SECTION 3403.1 OF THE BUILDING CODE STATES: "ADDITIONS OR ALTERATIONS TO ANY BUILDING OR STRUCTURE SHALL CONFORM WITH THE REQUIREMENTS OF THE CODE FOR NEW CONSTRUCTION. PORTIONS OF THE STRUCTURE NOT ALTERED AND NOT AFFECTED BY THE ALTERATION ARE NOT REQUIRED TO COMPLY WITH THE CODE REQUIREMENTS FOR A NEW STRUCTURE."

SECTION 3403.2 OF THE BUILDING CODE STATES: "ADDITIONS OR ALTERATIONS TO AN EXISTING STRUCTURE SHALL NOT INCREASE THE FORCE IN ANY STRUCTURAL ELEMENT BY MORE THAN 5 PERCENT, UNLESS THE INCREASED FORCES ON THE ELEMENT ARE STILL IN COMPLIANCE WITH THE CODE FOR NEW STRUCTURES, NOR SHALL THE STRENGTH OF ANY STRUCTURAL ELEMENT BE DECREASED TO LESS THAN THAT REQUIRED BY THIS CODE FOR NEW STRUCTURES. WHERE REPAIRS ARE MADE TO STRUCTURAL ELEMENTS OF AN EXISTING BUILDING, AND UNCOVERED STRUCTURAL ELEMENTS ARE FOUND TO BE UNSOUND OR OTHERWISE STRUCTURALLY DEFICIENT, SUCH ELEMENTS SHALL BE MADE TO CONFORM TO THE REQUIREMENTS FOR NEW STRUCTURES."

SECTION 3403.2.1 OF THE BUILDING CODE STATES: "WHERE AN EXISTING STRUCTURE HERETOFORE IS ALTERED OR REPAIRED, THE MINIMUM DESIGN LOADS FOR THE STRUCTURE SHALL BE THE LOADS APPLICABLE AT THE TIME OF ERECTION, PROVIDED THAT PUBLIC SAFETY IS NOT ENDANGERED THEREBY."

THESE CODE PROVISIONS HAVE BEEN INTERPRETED AS FOLLOWS:

1. THE BUILDING IS EXEMPT FROM A WIND OR SEISMIC ANALYSIS BECAUSE THE MAIN WIND (SEISMIC) FORCE RESISTING SYSTEM WILL NOT BE ALTERED DURING THIS RENOVATION.
2. ALL EXISTING GRAVITY ELEMENTS MUST BE CHECKED FOR DESIGN LOADS SHOWN ABOVE AND REINFORCED AS NECESSARY.
3. ALL DEFECTIVE STRUCTURAL ELEMENTS MUST BE REPAIRED OR REPLACED.

FOUNDATIONS

THE EXISTING BRICK MASONRY FOUNDATION WALL APPEARED TO BE CONSTRUCTED ON UNDISTURBED SOIL. BASED ON MASONRY WALL CRACKS, REINFORCEMENT OF EXISTING FOUNDATION IS REQUIRED.

NEW SLAB ON GRADE SHALL BE FOUNDED ON STABLE NATURAL SOIL OR CONTROLLED COMPACTED FILL.

ALL NEW FILL SHALL BE PLACED IN 8" MAXIMUM LOOSE LIFTS AND SHALL BE COMPACTED TO A MINIMUM OF 95 PERCENT MAXIMUM DRY DENSITY AS DETERMINED IN ACCORDANCE WITH ASTM D-698 (STANDARD PROCTOR METHOD). THIS REQUIREMENT SHALL BE INCREASED TO 98 PERCENT OF ASTM D-698 IN THE FINAL FOOT BENEATH FLOOR SLABS AND PAVEMENTS.

THE SURFACE AREA ADJACENT TO THE FOUNDATION WALL SHALL BE PROVIDED WITH ADEQUATE DRAINAGE AND SHALL BE GRADED SO AS TO DRAIN SURFACE WATER AWAY FROM FOUNDATION WALLS.

WHEN TOP OR SUBSOILS ARE EXPANSIVE, COMPRESSIBLE OR SHIFTING, SUCH SOILS SHALL BE REMOVED TO A DEPTH AND WIDTH SUFFICIENT TO ASSURE STABLE MOISTURE CONTENT IN EACH ACTIVE ZONE AND SHALL NOT BE USED AS FILL.

CONCRETE

CONCRETE SHALL BE PROPORTIONED, MIXED AND PLACED IN ACCORDANCE WITH ACI 318, "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE," AND ACI 301, " SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS." ANY ADMIXTURES MUST BE APPROVED BY THE STRUCTURAL ENGINEER.

MINIMUM 28 DAY COMPRESSIVE STRENGTH OF CONCRETE SHALL BE AS FOLLOWS:

SLABS-ON-GRADE	4000	PSI
TOPPING SLABS	4000	PSI

USE WATER REDUCING ADMIXTURES TO REDUCE WATER, INCREASE WORKABILITY AND DECREASE SHRINKAGE CRACKS. USE A MID-RANGE WATER REDUCING ADMIXTURE FOR SLABS AND A HIGH RANGE WATER REDUCING ADMIXTURE FOR POURED CONCRETE WALLS.

CRACK CONTROL JOINTS SHALL BE PLACED IN SLABS ON GRADE AT A MAXIMUM SPACING OF 12' UNLESS NOTED OTHERWISE.

SLAB-ON-GRADE CONSTRUCTION

PLACE FLOOR SLAB ON A WELL COMPACTED BASE. THE SUBGRADE SHALL BE GRANULAR, NON-EXPANSIVE SOIL (THAT IS, WITHOUT CLAY), WHICH HAS BEEN COMPACTED TO AT LEAST 95% AND VERIFIED BY ON-SITE TESTING.

SUPPORT THE 6 X 6 - W2.1 X W2.1 WELDED WIRE MESH AS REQUIRED TO INSURE THAT IT WILL BE LOCATED 1" FROM THE TOP OF SLAB.

CONCRETE STRENGTH SHALL BE 4000 PSI AT 28 DAYS. USE A WATER REDUCING ADMIXTURE TO REDUCE WATER, INCREASE WORKABILITY AND DECREASE SHRINKAGE CRACKS.

USE 6% AIR ENTRAINMENT ON EXTERIOR SLABS. DO NOT USE AIR ENTRAINMENT ON INTERIOR SLABS (3% MAXIMUM AIR ENTRAINMENT).

THE CONTROL JOINT SPACING SHALL BE APPROXIMATELY 12' FOR A 4" THICK SLAB. PLACE CONTROL JOINTS TO AVOID REENTRANT CORNERS. MAKE SAWCUTS TO FORM WEAKEN PLANE CONTROL JOINTS AS SOON AS POSSIBLE.

LIGHTLY DAMPEN THE SUBGRADE BEFORE PLACING CONCRETE TO PREVENT THE SUBGRADE FROM ABSORBING WATER FROM CONCRETE MIX. APPLY WATER AT NEARLY THE SAME RATE IT SOAKS INTO THE SUBGRADE SURFACE.

STEEL TROWEL THE CONCRETE TO A SHINY FINISH WHICH RESULTS IN A HARD, DENSE SURFACE.

BRICK MASONRY

NEW BRICK MASONRY SHALL BE OF A QUALITY AT LEAST EQUAL TO THAT REQUIRED BY ASTM SPECIFICATIONS (C62). THE GRADE OF UNITS MUST BE 3,000 PSI (MAX AND MIN).

NEW MORTAR SHALL BE OF A QUALITY AT LEAST EQUAL TO THAT REQUIRED BY ASTM "STANDARD SPECIFICATIONS FOR MORTAR FOR UNIT MASONRY" (C270). USE TYPE "N" MORTAR.

EXPANSION AND EPOXY ANCHORS

THE CONTRACTOR MAY SELECT THE BRAND(S) OF ANCHORS TO USE ON THIS PROJECT, BUT THE ANCHORS MUST MEET THE MINIMUM LOAD REQUIREMENTS SHOWN IN THE TABLES BELOW. NOTE THAT THESE LOADING REQUIREMENTS ARE BASED UPON MINIMUM EDGE AND SPACING DISTANCES. IF A SMALLER EDGE OR SPACING DIMENSION IS SHOWN ON THE STRUCTURAL DRAWINGS, THE STRUCTURAL ENGINEER HILL HAVE USED A "LOAD ADJUSTMENT FACTOR" ON THE ANCHOR TO ACCOUNT FOR THE REDUCED DIMENSION.

EPOXY ANCHORS IN CONCRETE

SIZE	EMBED	SHEAR	TENSION	EDGE DIST	SPAC
1/2"	6 1/2"	1435#	3755#	8"	9"
5/8"	7 1/2"	3025#	5870#	10"	10"
3/4"	10"	4335#	8455#	14"	14"

EPOXY ANCHORS IN MULTI-WYTHE BRICK

SIZE	EMBED	SHEAR	TENSION	EDGE DIST	SPAC
1/2"	10"	930#	1270#	16"	8"
5/8"	10"	1355#	1285#	16"	8"
3/4"	13"	1800#	2100#	16"	8"

SOLID WOOD FRAMING, HEADERS AND PLYWOOD

ALL SOLID WOOD FRAMING SHALL COMPLY WITH THE NATIONAL FOREST PRODUCTS ASSOCIATION "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION."

NEW INTERIOR STUD FRAMING SHALL BE THE GRADE AND SPECIES SHOWN ON THE STRUCTURAL DRAWINGS.

ALL MEMBERS SHALL BE FRAMED, ANCHORED, TIED AND BRACED IN ACCORDANCE WITH GOOD CONSTRUCTION PRACTICE AND THE NORTH CAROLINA STATE BUILDING CODE.

STRUCTURAL STEEL

FABRICATE AND ERECT ALL NEW STRUCTURAL STEEL IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL OF BUILDINGS," WHERE THE MATERIAL USED CONSISTS OF PLATES, SHAPES OR BARS.

THE STEEL USED SHALL HAVE THE FOLLOWING MINIMUM YIELD STRESS:

STRUCTURAL STEEL WIDE FLANGE	50 KSI
STRUCTURAL PIPE COLUMNS	35 KSI
STRUCTURAL TUBE COLUMNS	46 KSI
MISCELLANEOUS SHAPES, BARS AND PLATES	36 KSI

BEAMS AND LINTELS SHALL BEAR ON 8" MINIMUM OF MASONRY UNLESS OTHERWISE NOTED.

USE 3/4" DIAMETER A-325 BOLTS FOR ALL STEEL TO STEEL CONNECTIONS U.N.O. BOLTS SHALL BE TIGHTENED TO A SNUG TIGHT CONDITION U.N.O. THE SNUG TIGHT CONDITION IS DEFINED AS THE TIGHTNESS THAT EXISTS WHEN ALL PLIES OF A CONNECTION ARE IN FIRM CONTACT.

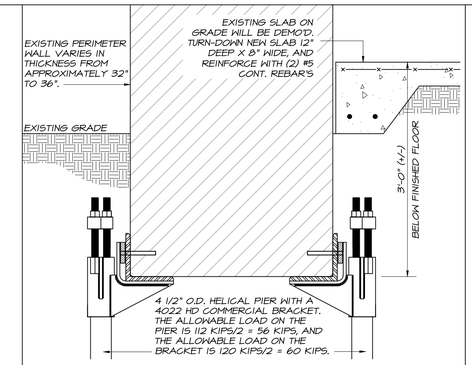
USE A36 STEEL FOR ALL ANCHOR BOLTS U.N.O.

USE E-70 ELECTRODES FOR ALL SHOP AND FIELD WELDING.

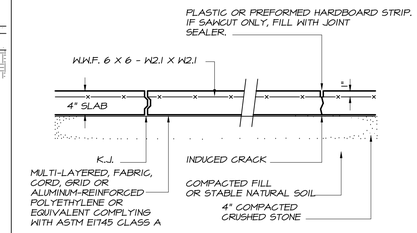
THE STEEL FABRICATOR SHALL DESIGN ALL STEEL TO STEEL CONNECTIONS FOR THE REACTIONS SHOWN ON THE DRAWINGS USING STANDARD CONNECTION DETAILS AS ILLUSTRATED. HOWEVER, CONNECTIONS FOR WIDE FLANGE BEAMS SHALL HAVE THE FOLLOWING MINIMUM ROWS OF BOLTS:

W8, W10, W12 BEAMS	(2) ROWS OF BOLTS
W14, W16 BEAMS	(3) ROWS OF BOLTS

FOR MISCELLANEOUS STEEL NOT SHOWN ON THESE DRAWINGS, SEE ARCHITECTURAL AND OTHER ENGINEERING DRAWINGS.



01 FOUNDATION UNDERPINNING DETAIL
NOT TO SCALE

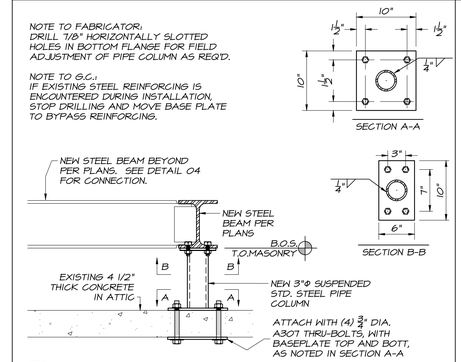


NOTES:

USE A KEYED CONSTRUCTION JOINT (K...I) BETWEEN POURS.

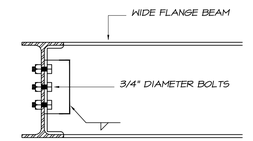
SLAB IS DESIGNED FOR COMMERCIAL OCCUPANCY WITH A MAXIMUM LIVE LOAD OF 100 PSF AND NO LARGE CONCENTRATED LOADS.

02 TYPICAL NEW SLAB ON GRADE
NOT TO SCALE



03 BEAM AND COLUMN CONNECTION IN ATTIC
NOT TO SCALE

BEAM SIZE	BOLT ROWS
8" WF	2
10" WF	2

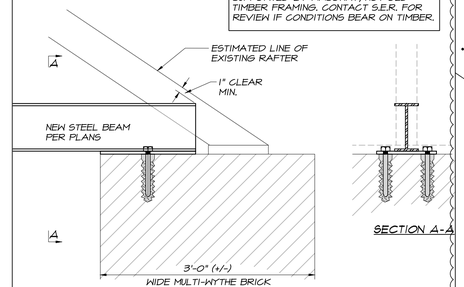


NOTE

SEE FRAMING PLAN FOR BEAM SIZES.

04 BEAM TO BEAM CONN
NOTE TO CONTRACTOR
NOT TO SCALE

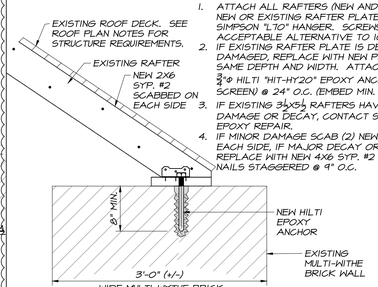
NOTE TO CONTRACTOR
FIELD VERIFY BEARING PLATE IS FULLY SUPPORTED BY MASONRY, NOT OLD TIMBER FRAMING. CONTACT S.E.R. FOR REVIEW IF CONDITIONS BEAR ON TIMBER.



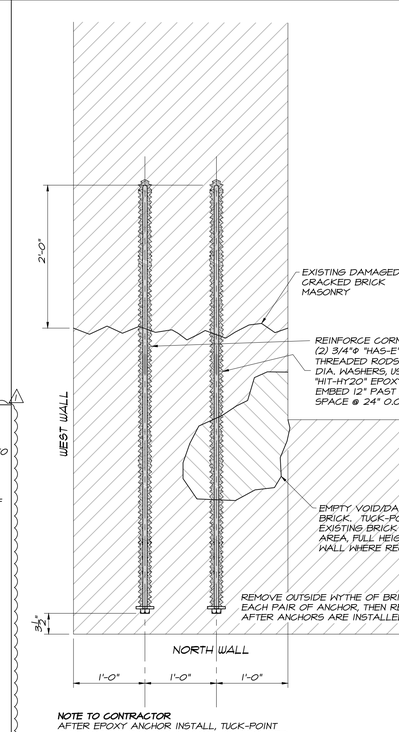
06 NEW STEEL BEAM BEARING
NOT TO SCALE

NOTES

1. ATTACH ALL RAFTERS (NEW AND EXISTING) TO NEW OR EXISTING RAFTER PLATE WITH SIMPSON "LTO" HANGER. SCREWS ARE AN ACCEPTABLE ALTERNATIVE TO 10d NAILS
2. IF EXISTING RAFTER PLATE IS DECAYED OR DAMAGED, REPLACE WITH NEW P.T. PLATE OF SAME DEPTH AND WIDTH. ATTACH WITH NEW 3/4"ø HILTI "HIT-HY20" EPOXY ANCHOR (WITH SCREEN) @ 24" O.C. (EMBED MIN. 8")
3. IF EXISTING 3 1/2"X5 1/2" RAFTERS HAVE MINOR DAMAGE OR DECAY, CONTACT S.E.R. FOR EPOXY REPAIR.
4. IF MINOR DAMAGE SCAB (2) NEW 2X6, (1) ON EACH SIDE, IF MAJOR DECAY OR DAMAGE, REPLACE WITH NEW 4X6 SYP. #2 WITH 10d NAILS STAGGERED @ 9" O.C.

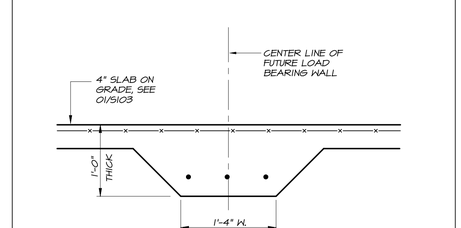


07 RAFTER CONNECTION DETAIL
NOT TO SCALE



NOTE TO CONTRACTOR
AFTER EPOXY ANCHOR INSTALL, TUCK-POINT AND MORTAR EXISTING CRACKS AS REQUIRED.

05 BRICK REINFORCEMENT AT CORNER OF WEST ELEVATION
NOT TO SCALE



NOTES

THE THICKENED SLAB MUST REST ON FIRM UNDISTURBED, OR COMPACTED, SOIL.

REINFORCE THE THICKENED SLAB WITH (3) #5 CONTINUOUS BARS, TYPICAL.

08 THICKENED SLAB @ LOAD BRNG WALL
NOT TO SCALE

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Planning
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SEALS

NORTH CAROLINA REGISTERED SEAL 7903 ENGINEER CHARLES A. LYSAGHT 11/13/12

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PROJECT

OLD CURRITUCK JAIL AND COURTHOUSE RESTORATION PHASE ONE
Currituck County, North Carolina

CLIENT

COUNTY OF CURRITUCK

DRAWING TITLE
STRUCTURAL DETAILS

REVISIONS

NUMBER	DESCRIPTION	DATE
1	RAFTER REPAIR DETAIL	02/22/13

DRAWN BY GTH
CHECKED BY CAL
DATE ISSUED 10.31.2012

S103
3 OF 3